MECHANICAL BEHAVIOR OF THE DIFFERENT SOILS UNDER THE SOIL- PILE INTERACTION

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**ABSTRACT**

The study focuses on the nonlinear interaction between soil, pile, and pile cap, particularly examining the behavior of soil and pile under working load conditions. The research utilizes ANSYS software to model an 8-story plane frame and a 2D plane of soil and pile, employing Beam188 and Plane 183 elements for the analysis. The beam-columns in the model are sized at 300mm x 300mm, while the piles have a diameter of 400mm with a pile cap width of 2m and thickness of 300mm. The study considers soil of medium stiffness with three different moduli of elasticity: E-15, E-35, and E-65. The frame is subjected to a combination of dead load, live load (40 kN/m), and seismic load according to IS:1893 (Part I):2002 for zone V. Finite Element Analysis (FEA) with contact surface pairing is used to evaluate the interaction between soil and pile. The analysis results are presented in terms of X-Displacement, Y-Displacement, von Mises stress contour, contact pressure, and their contact status. The study concludes that soil with a Young's modulus of elasticity E-35 demonstrates greater stiffness compared to soils with E-15 and E-65. These findings are illustrated graphically and through contour diagrams for each soil type. Finally, the study discusses the future scope of the project and provides references for further research.

**Key Words:** Non linear interaction, soil and pile, pile cap, ANSYS, stiffness and Finite Element Analysis.

 **Introduction**

In case of the civil engineering structures foundation involves with direct contact with Soil.The foundation of a structure resting on a settable soil mass under goes differential settlement which alters the forces in the structural elements significantly. In case of analysis of multistory building, the seismic loads and wind loads are necessary to take as design loads and the building frame, foundation and soil mass are considered to act as single, compatible structural unit. The linear or nonlinear behavior of soil mass is main cause of differential settlement which redistributes the forces in the elements of the structure mainly in the beams and column. When the external forces such as earthquake act on these systems, the structural displacements and the ground displacements both are independent of each other. The processing which the response of the soil affects the motion of the structure and the motion of the structure affects the response of the soil is termed as Soil-Structure Interaction (SSI).

**Mmotivation Behind the study**

The building frame and its foundation along with the soil on which it rests, together constitute a complete structural system. In the conventional analysis, a structure is analyzed as an independent frame assuming unyielding supports and the interactive response of soil foundations disregarded. This kind of analysis does not provide realistic behavior and sometimes may cause failure of structure. Thus it is essential to consider the soil-structure interaction effect especially in case of high rise buildings. The resulting differential settlements of soil mass are responsible for the redistribution of forces.

**Literature Survey**

**Francesco Cavalieri et al (2020)** Dynamic Soil–Structure Interaction (SSI) involves the complex interaction between the soil, the foundation, and the structure, which is particularly challenging to model accurately, especially when considering soil nonlinearity. This paper assesses how different SSI models impact the seismic fragility functions of unreinforced masonry (URM) buildings on shallow foundations. The study concludes that different SSI models, particularly when accounting for soil nonlinearity, significantly influence the seismic fragility functions of URM buildings on shallow foundations. This highlights the importance of selecting appropriate SSI models in seismic assessments to ensure realistic predictions of structural performance during earthquakes. By comparing the linear substructure models with the nonlinear approach, the study evaluates how assuming linearity or nonlinearity in the soil affects the fragility assessment. The sensitivity of fragility functions to different SSI assumptions provides insights into the significance of accurately modeling soil behavior for seismic risk assessments. These are probabilistic tools used to describe the likelihood of reaching or exceeding different levels of structural damage under varying seismic intensities. This simplification is crucial for focusing the analysis on the impact of SSI rather than the complexities of the superstructure itself.

[**Aldo Fernández Limés**](https://www.researchgate.net/profile/Aldo-Fernandez-Limes?utm_content=businessCard&utm_source=publicationDetail&rgutm_meta1=AC%3A25435234&_tp=eyJjb250ZXh0Ijp7ImZpcnN0UGFnZSI6InB1YmxpY2F0aW9uIiwicGFnZSI6InB1YmxpY2F0aW9uIn19) **et al (2023)** Soil-structure interaction (SSI) is a critical field of study within civil and structural engineering, with a rich body of literature detailing its effects on various types of buildings and structures. While the primary motivation for investigating SSI has traditionally been its impact during seismic events, it is crucial to acknowledge that other sources of excitation, such as wind forces, vibrations from machinery, and large static loads, can also significantly affect structures when conditions for SSI are present. The primary aim of this text is not to provide an exhaustive overview of all existing knowledge on SSI. Instead, it seeks to underscore the importance of SSI effects on different structures through illustrative case studies. These case studies demonstrate the consequences of SSI on structural response and present methods for analyzing these effects. Special emphasis is placed on non-seismic excitations, an area that, despite being equally significant, has not received as much attention as seismic studies. The text provides case studies that highlight the real-world implications of SSI under non-seismic excitations. These cases illustrate how neglecting SSI can lead to significant structural issues and demonstrate the benefits of incorporating SSI analysis into the design process. This work aims to inspire engineers and researchers to integrate SSI analysis into their projects more consistently, particularly for non-seismic excitations. By doing so, they can enhance the safety, performance, and longevity of structures. The text serves as both an educational resource and a motivational tool, encouraging the consideration of SSI in all relevant structural analyses.

 **Methodology and Finite Element Analysis on soil-Pile Interaction**

The methodology adopted in modeling and analysis of an eight storey building frame has been discussed in this chapter. ANSYS software is used for analyzing the frame with and without considering structure-soil interaction. The overview of the program in the form of flow chart has been detailed.

**Geometrical Properties**

**Table -1Geometrical properties of the super structure and foundation**

|  |  |  |
| --- | --- | --- |
| **S.No** | **Structural components** | **Properties and size of the components** |
| 1. | All floor and plinth beam | 0.3m\*0.3m |
| 2. | Columns | 0.3m\*0.3m |
| 3. | Footings | 2m\*2m\*1m |
| 4. | Number of bays | 3 |
| 5. | Number of storeys | 8 |
| 6. | Floor beam and plinth beamuniformly distributed loading | 40 kn/m |
| 7. | Depth of soil | 5.0 m |

**Material Properties** The material of super-structure and foundation i.e. concrete is considered to behave in linearelastic manner. Table 2 shows the material properties of the structure and soil.

**Table -.2 Material properties of the structure and soil**

|  |  |  |
| --- | --- | --- |
| **S. No.** | **Structural components** | **Properties and size of****components** |
| 1. | Modulus of elasticity ofconcrete (N/mm2) | 2.17 x 107 kN/m2 |
| 2. | Poisson’s ratio of concrete | 0.15 |

 **Properties of soil**

The elastic constants for different types of soil used in interaction analysis are given in Table3

**Table -3 Soil Elastic Constant**

|  |  |  |  |
| --- | --- | --- | --- |
| **Soil****Type** | **Soil****Designation** | **Modulus of****elasticity****(kN/m2)** | **Poisson****ratio** |
| Hard | E-65 | 65000 | 0.35 |
| MediumHard | E-35 | 35000 | 0.4 |
| Soft | E-15 | 15000 | 0.4 |

**Description of the Elements**

To carry out the nonlinear interaction analysis Beam 188 element is used to model the frame and Plane 183 element is used to model the soil, pile and its pile cap .Three types of soil having different modulus of elasticity E-65, E-35and E-15 are considered. In this analysis to carry out the non-linear analysis contact between the soil and pile is created and the results of the analysis is in the form of Von mises stress contours, Displacement along X direction and Displacement along Y direction, Contact Pressure, and Contact status.



**Figure 1.Geomatrical details of the frame under analysis**

To carry out the nonlinear interaction analysis Beam 188 element is used to model the frame and Plane 183 element is used to model the soil, pile and its pile cap. Three types of soil having different modulus of elasticity E-65, E-35and E-15 are considered.



**Figure 2 Discredited model of frame for NLIA**

**Result Discussion and Conclusion**

In this chapter the results of analysis are discussed for the four cases NLIA-65, NLIA-35 and NLIA-15. The main outcomes are:

1. Comparison of displacements in X-direction at different footings.
2. Comparison of displacements in Y-direction at different footings.
3. Comparison of Vonmises at different footings.
4. Comparison of Contact pressure at different footings.

**Table 4 X-Direction Displacements**

|  |  |  |  |
| --- | --- | --- | --- |
| **Type of Soil** | **E-65** | **E-35** | **E-15** |
| **X-Displacement in (mm)** | 0.15 | 0.05 | 1.85 |

**Table 45Y-Direction Displacements**

|  |  |  |  |
| --- | --- | --- | --- |
| **Type of Soil** | **E-65** | **E-35** | **E-15** |
| **Y-Displacement in (mm)** | 0.00925 | 0.01525 | 0.245 |

**Table 6 Vonmisses stress**

|  |  |  |  |
| --- | --- | --- | --- |
| **Type of Soil** | **E-65** | **E-35** | **E-15** |
| **Vonmises (Mpa)** | 14.62 | 1.82 | 57.90 |

**Table 7 Contact stresses**

|  |  |  |  |
| --- | --- | --- | --- |
| **Type of Soil** | **E-65** | **E-35** | **E-15** |
| **ContactStresses (Mpa)** | 5.35 | 0.60 | 14.62 |

 **Conclusions made from the observation**

* Maximum displacement in x-direction and in Y-direction is on soil having modulus of elasticity E-15
* Contact stresses of soil E-65 are 16% higher than E-35 soil and Contact stresses of E-15 soil are 2.73 times higher than E-65 soil.
* Vonmisses stresses of E-65 are 8 times higher than E-35 soil and vonmisses stresses of E-15 soil are 4 times higher than E-64 soil.
* Soil E-35 shows more stiffness than E-65 and E-15.

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