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#### PLANNING, ANALYSIS AND DESIGN OF HOSPITAL BUILDING BY USING ETABS

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#### ABSTRACT

In every aspect of human civilization we need building structures to live in or to get what we need. But it is not only building structures but to build efficient structures so that it can fulfil the main purpose for what it is made for. Here comes the role of civil engineering and more precisely the role of analysis of structure. The present design consists of G+5 commercial building. The floor to floor distance is 3m. There are many classical methods to solve the design problems and with time new software's are also coming into play. Here in this project work based on software named "ETABS", design and analysis of multistoried G+6 building using RCC structure are carried out. ETABS are the effective software's which are used for the purpose of analysis and design of structure by the structural engineers. These software's give more precise and accurate results than manual techniques. ETABS was used to create the mathematical model of the Burj Khalifa (UAE). The input, output and numerical solution techniques of ETABS are specifically designed to take advantage of the unique physical and numerical characteristics associated with building type structures. ETABS provides both static and dynamic analysis for wide range of gravity, thermal and lateral loads. In this project I analyze and design the multi-storey building using ETABS. Comparing results obtained by software's and justifying economical one or to decide best software to adopt for design and to increase the number of storey of the structure.

Keywords -Auto CAD, Code, Etabs

#### 1. INTRODUCTION

India is a developing country and has developed in every sense since its independence. The most important are the infrastructural development which is to be provided in order to generate an efficient way to work. Infrastructure has become very important in today's era, since it has been the key factor in the growth of a country, be it in information technology, logistics or any other senses. The presence of high rise buildings in a country displays its economic strength. Now in India in cities like Mumbai, the government is sanctioning the construction of highrise buildings. As of next year, Mumbai will be the city having the world's tallest residential building named World one. Building these mega structures has become common in today's world since it is the only way in which humans can make the maximum use of the piece of land. But the main concern is when the designing part of these high rise structure comes into play. Unlike the old times, the designing of the building was done manually. Until the innovation into civil engineering field which started the use of computer-oriented design software into the process of designing of a structure or a high rise building. Today there are hundreds of software which consult into the designing of the civil related structures and also managerial software which deals with the management of the construction activities on the site. The various software in the field of designing are namely E-Tabs, STAAD.pro, SAFE etc. These are most widely used software around the world. The main focus being on E-Tabs software. Since in India, it is been used from recent past even though it has been existing since last three decades. The software ETABS has an ultimate integrated system which helps in computing the values required for the designing and analysis of the building. It has been used in some of the most renowned building projects the mankind has known. E-tabs was used to create the mathematical modelling of the Burj Khalifa, currently the tallest building the world, designed by Chicago, Illinois-based Skidmore, Owings and Merrill (SOM). Various factors like gravity, wind and seismic response were all characterized using ETABS. ETABS is commonly used to analyse skyscrapers, parking garages, steel and concrete structures, low-rise buildings, portal frame structures, and high rise buildings. Concrete structures provide a vast availability in the field of civil engineering. It has redefined the construction methods and building high-rise structures at easy. Concrete structures also include various structures like Dams, retaining walls, bridges and skyscrapers. After the introduction of concrete cement in before Christ era, it has been in the construction field since then. It is the basic requirement for the building of any structure which is of vital importance. In India concrete structures are used almost everywhere, even the simple residential blocks are concrete made unlike in western countries where the houses are made of wood. Concrete has been the most favourable material in the field of @International Journal Of Progressive Research In Engineering Management And Science Page | 890



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construction because of its properties as it is durable, strong, resistant and reliable. It is used under various types of Reinforced Cement Concrete, plain cement concrete, and various under the same categories. The necessity and importance of concrete are immense. To analyse the structural details of the structure.

#### 2. LITERATURE REVIEW

**Varalakshmi V et.al** (2014) [1] built the skeleton of a G+5 home from the ground up. IS 875(Part I & II)-1987 dead and live loads and IS 1986-1985 HYSD bars were employed in this study. They observed that a reinforced concrete structure's safety depends on its structural analysis, design, and reinforcing characteristics.

**Chandrashekar et.al** (2015) [2] ETABS was used for both the assessment and planning of the multi-story structure. A G+5 building's response to lateral wind and seismic loads was analysed in ETABS. They considered fire spread and the best fireproof material. They offered the innovative and user-friendly ETABS application for high-rise structures to save design time.

**Balaji.U** and Selvarasan M.E (2016) [3] multi-story structures' static and dynamic loads were analysed and designed using ETABS. The effects of an earthquake on a G+13 house were analysed using ETABS. For both static and dynamic analyses, linear material characteristics were used. Behaviour in areas of high seismicity and soil type II was analysed using a non-linear model. Base shear and displacements were charted and analysed.

**Geethu et.al** (2016) [4] compared STAAD.Pro with ETABS multi-story building analysis and design. They designed residential and commercial buildings. The Auto CAD-drafted plan followed the national construction code. ETABS software had greater bending moment and axial force readings.

**Nagaratna et.al** Multi-storey structural analysis and design project RCC building analyses multistorey buildings using structural analysis methodologies and software (ETABS). E-TABS software analyses beams and columns, while IS: 456-2000's "LIMIT STATE METHOD" designs slabs and footings. Uses M-25 concrete and Fe-415 steel.

**Maruthi T et.al** In Civil Engineering, a building is a structure with many parts such a base, ETABS' input, output, and Building-type structures' physical and numerical properties are used to solve numerical problems. Architectural features include walls, columns, floors, roofs, doors, windows, ventilators, stairlifts, surface treatments, etc. Structural studies and design create a structure that can withstand all loads.

**B.** Anusha et.al. We understand the housing issues in this era of diminishing space, time, and expectation. Any institution now offers student hostels. G+4 building with stairs and elevator accommodates 120 students. The model is being prepared by ETABS. It is capable of handling complex structures as well as high-rise skyscrapers. It performs assessments on beams, columns, slabs, shear walls, and other structural elements. We are able to quickly analyse a wide variety of materials using ETABS, including concrete, steel, reinforced concrete, and others. Produce gravitational, lateral, and self-weights on its own automatically. (Varalakshmi V et al-2014).

**Chandrakala V B et.al**, Civil engineering constructions have foundations, walls, columns, floors, roofs, doors, windows, ventilators, stairs lifts, and surface treatments. Design and analysis create load-bearing structures. Structural analysis and design need geotechnical study. Geotechnical site investigations design and install foundations. Structural engineers must carefully design the most cost-effective and functional building for its intended application.

**Nilendu Chakrabortty et.al,** The successful plan and development of seismic tremor safe structures have a lot more significance worldwide, so designs are trying to use various materials to their best advantage, keeping in mind the unique properties of each material. Combining the most desirable qualities of a single material with the functional and aesthetic requirements of the project results in the construction of the most robust and aesthetically pleasing buildings.

**Nirmal S. et.al** ETABS engineering software analyses and designs multi-storey buildings. Extended Three- Dimensional Building System Analysis is ETABS. CAD drawings may be used to create ETABS models or as templates for ETABS items. Software generates full reinforcement report. Building floor levels are identical, reducing modelling and design time. Similar storey model creation for speed. Structural components might be steel, RCC, composite, or user defined.

**Shah H. J. and Jain S. K. et.al,** Have examined building earthquake performance utilising various load combinations. They also examined high-wind building performance. They also detailed structural components utilising Indian Standard Code. Restoring force depends on shear wall location.

Uma M and Nagarajan (2016) et.al, Changed the shear wall placement to get the best multistorey building structural layout. The shear wall at the building corner has minimal displacement and drifts, making it the best site. Shear wall material also affects building performance.

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#### 3. METHODOLOGY

Stilt, 1st, 2nd, and one more level are planned for the residential construction. RCC beams and slabs support the structural framework. Columns are spaced for stability and utility with

#### RCC shear walls.

Architectural planning and serviceability determine beam planning. Secondary beams are required.

The system resists the entire design force according to their lateral stiffness considering floor level interaction. IS:1893(Part1)-2016 classifies the building as an SMRF.

Horizontal diaphragms are floor slabs.

3-D ETAB Software will be utilised for structural system analysis and design/detail. All designs must adhere to Indian requirements.

#### PLAN OF THE PROJECT



#### LINE DIAGRAM



#### **DETAILS OF THE PROJECT**

- ≻ Structure: G+5
- ➤ Floor height(First Floor to Fifth Floor) : 4.0m
- ➢ Grade of Concrete : M-25
- ➢ Unit weight of Concrete : 25 KN/M3
- ▶ Rebar material grade : HYSD500
- ➢ Beam sizes :B1(300x230)mm
- ▶ B2(300X450)mm
- ► B3(300X600)mm
- ➢ Column sizes : C1(350x350)mm
- ➤ C2(450X450)mm
- ≻ C3(500x500)mm
- ➢ PLOT AREA:- 2404 sqm
- ➤ CARPET AREA :-1416 sqm
- ➤ SUPER BUILT UP AREA :-1422 sqm
- > DEAD LOAD (IS875-part-1) Unit weight of building materials.
- ▶ LIVE LOAD (IS875- PART-2)
- ➢ WIND LOAD(IS875-part-3)
- SEISMIC LOAD(IS1893-part-1)
- LOAD COMBINATIONS



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**3D VIEW** 



#### 4. ANALYSIS AND RESULTS

The present structure is modelled and analyzed and analysis using ETABS. For the analysis of gravity load and seismic loads. The live load of the structure is considered 2 kN/m2. For the lateral load analysis (earthquake) parameter are considered as per Indian code basis.

#### For area of longitudinal reinforcement, go to design menu in ETAB, concrete frame design and start design check.



SHEAR FORCE



#### BENDING MOMENT





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Design > Concrete Frame Design > Display Design Information shows steel percentages and column beam steel areas.



#### BEAM-1 ETABS Concrete Frame Design

#### IS 456:2000 + IS 13920:2016 Beam Section Design



Beam Element Details Type: Ductile Frame (Summary)

Level	Element	Unique Name	Section ID	Combo ID	Station Loc	Length (mm)	LLRF
Story6	B61	26	BEAM	DCon19	3200	3375	1

Section Properties

b (mm)	h (mm)	<b>b</b> <sub>f</sub> (mm)	d <sub>s</sub> (mm)	<b>d</b> <sub>ct</sub> ( <b>mm</b> )	<b>d</b> <sub>cb</sub> ( <b>mm</b> )
230	350	230	0	35	35

Material Properties

E <sub>c</sub> (MPa)	f <sub>ck</sub> (MPa)	Lt.Wt Factor (Unitless)	f <sub>y</sub> (MPa)	f ys (MPa)
25000	25	1	500	500

Design Code Parameters



Factored Forces and Moments

Factored	Factored	Factored	Factored
<b>M</b> u3	T u	<b>V</b> u2	P u kN
kN-m	kN-m	kN	
-28.2484	2.7973	30.0286	5.734

Design Moments, M <sub>u3</sub> & M <sub>t</sub>

FactoredFactoredMoment kN-mM t kN-m		Positive Moment kN-m	Negative Moment kN-m	
-28.2484	4.1495	0	-32.3979	

Design Moment and Flexural Reinforcement for Moment, M  $_{u3}$  & T  $_u$ 

	Design Moment kN-m	Design Moment kN-m	-Moment Rebar mm²	+Moment Rebar mm²	Minimum Rebar mm²	Required Rebar mm²
Top (+2 Axis)	-32.3979		241	0	241	174
Bottom (-2 Axis)		0	121	0	0	121



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Shear Force and Reinforcement for Shear, V  $_{u2}\,\&\,T$   $_u$ 

Shear V <sub>e</sub> kN	Shear V <sub>c</sub> kN	Shear V <sub>s</sub> kN	Shear V <sub>p</sub> kN	Rebar A <sub>sv</sub> /s mm²/m
42.8968	0	62.3563	23.8886	548.55

Torsion Force and Torsion Reinforcement for Torsion,  $T_u\,\&\,V_{\,\,U2}$ 

T u kN-m	V u kN	Core b 1 mm	Core d 1 mm	Rebar A <sub>svt</sub> /s mm²/m
2.7973	30.0286	180	300	254.5

BEAM -2

ETABS Concrete Frame Design

IS 456:2000 + IS 13920:2016 Beam Section Design



Beam Element Details Type: Ductile Frame (Summary)

Level	Element	Unique Name	Section ID	Combo ID	Station Loc	Length (mm)	LLRF
Story6	B27	157	FSec2	DCon21	8080	23360	1

Section Properties

b (mm)	h (mm)	<b>b</b> <sub>f</sub> ( <b>mm</b> )	d <sub>s</sub> (mm)	<b>d</b> <sub>ct</sub> ( <b>mm</b> )	<b>d</b> <sub>cb</sub> ( <b>mm</b> )
300	450	300	0	30	30

Material Properties

E <sub>c</sub> (MPa)	f <sub>ck</sub> (MPa)	Lt.Wt Factor (Unitless)	f <sub>y</sub> (MPa)	f ys (MPa)
27386.13	30	1	500	500

Design Code Parameters

γC	<b>¥</b> s
1.5	1.15

Factored Forces and Moments

Factored	Factored	Factored	Factored
<b>M</b> u3	T u	<b>V</b> u2	P u kN
kN-m	kN-m	kN	
-180.3254	84.196	119.4508	28.2329

Design Moments, M <sub>u3</sub> & M <sub>t</sub>

Factored Moment kN-m	FactoredFactoredMoment kN-mM tkN-m		Negative Moment kN-m	
-180.3254	123.8176	0	-304.143	



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#### Design Moment and Flexural Reinforcement for Moment, M u3 & T u

	Design Moment kN-m	Design Moment kN-m	-Moment Rebar mm²	+Moment Rebar mm²	Minimum Rebar mm²	Required Rebar mm <sup>2</sup>
Top (+2 Axis)	-304.143		2019	0	2019	648
Bottom (-2 Axis)		0	648	0	0	648

Shear Force and Reinforcement for Shear, V u2 & T u

*Shear V <sub>e</sub> kN Shear V <sub>c</sub> kN		Shear V <sub>s</sub> kN	Shear V <sub>p</sub> kN	Rebar A <sub>sv</sub> /s mm²/m	
O/S #45	0	0	63.7662	0	

Torsion Force and Torsion Reinforcement for Torsion,  $T_u\,\&\,V_{\,\,U2}$ 

T u kN-m	V u kN	Core b 1 mm	Core d 1 mm	Rebar A <sub>svt</sub> /s mm²/m
84.196	119.4508	260	410	3030.07

O/S #45 Shear stress due to shear force and torsion together exceeds maximum allowed

#### BEAM-3

ETABS Concrete Frame Design

#### IS 456:2000 + IS 13920:2016 Beam Section Design



Beam Element Details Type: Ductile Frame (Summary)

Level	Ele	ment	Unique	Name	Section I	D	Combo ID	S	Station Loc	Lengtl	n (mm)	LLRF
Story6	В	29	16	59	FSec3		DCon2		43660	579	970	1
b (mn	n)	h	(mm)	<b>b</b> <sub>f</sub>	(mm)		<b>d</b> <sub>s</sub> ( <b>mm</b> )		<b>d</b> <sub>ct</sub> (m	m)	d <sub>cb</sub>	(mm)
300			600		300		0		30			30

E c (MPa)	f ck (MPa)	Lt.Wt Factor (Unitless)	fy(MPa)	f ys (MPa)
27386.13	30	1	500	500

**Design Code Parameters** 

<b>ү</b> С	<b>¥</b> s
1.5	1.15

#### Factored Forces and Moments

Factored	Factored	Factored	Factored
<b>M</b> u3	T u	<b>V</b> u2	P u kN
kN-m	kN-m	kN	
-348.4219	66.3015	155.4774	32.622



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Design Moments, M u3 & M t

Factored Moment kN-m	Factored M t kN-m	Positive Moment kN-m	Negative Moment kN-m
-348.4219	117.0027	0	-465.4246

Design Moment and Flexural Reinforcement for Moment, M  $_{u3}\,$  & T  $_u$ 

	Design -Moment kN-m	Design +Moment kN-m	-Moment Rebar mm <sup>2</sup>	+Moment Rebar mm²	Minimum Rebar mm²	Required Rebar mm²
Top (+2 Axis)	-465.4246		2171	0	2171	543
Bottom (-2 Axis)		0	543	0	0	543

Shear Force and Reinforcement for Shear, V  $_{u2}\$  & T  $_{u}$ 

Shear V <sub>e</sub>	Shear V <sub>c</sub>	Shear V <sub>s</sub>	Shear V <sub>p</sub>	Rebar A <sub>sv</sub> /s
kN	kN	kN	kN	mm²/m
155.4774	124.2947	384.7908	37.4705	1870.68

Torsion Force and Torsion Reinforcement for Torsion,  $T_u \& V_{U2}$ 

T u kN-m	V u kN	Core b 1 mm	Core d 1 mm	Rebar A <sub>svt</sub> /s mm²/m
66.3015	155.4774	260	560	1870.68

COLUMN-1

ETABS Concrete Frame Design

IS 456:2000 + IS 13920:2016 Column Section Design



Column Element Details Type: Ductile Frame (Summary)

Level	Element	Unique Name	Section ID	Combo ID	Station Loc	Length (mm)	LLRF
Story1	C40	608	COLUMN	DCon21	0	3000	0.851

Section Properties

b (mm)	h (mm)	dc (mm)	Cover (Torsion) (mm)
230	350	55	25

Material Properties

E <sub>c</sub> (MPa)	f <sub>ck</sub> (MPa) Lt.Wt Factor (Unitless) f <sub>y</sub> (MPa)	f ys (MPa)
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25000 25 1

500 500

Design Code Parameters

γC	¥s
1.5	1.15



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Axial Force and Biaxial Moment Design For P  $_{\rm u}$  , M  $_{\rm u2}$  , M  $_{\rm u3}$ 

Design P <sub>u</sub> kN	Design M <sub>u2</sub>	Design M <sub>u3</sub>	Minimum M <sub>2</sub>	Minimum M 3	Rebar Area	Rebar %
	kN-m	kN-m	kN-m	kN-m	mm²	%
-67.1618	74.2438	22.2888	1.3432	1.3432	3481	4.32

Axial Force and Biaxial Moment Factors

	K Factor Unitless	Length mm	ial Moment kN-m	tional Moment kN-m	imum Moment kN-m
Major Bend(M3)	1	2500	8.9155	0	1.3432
Minor Bend(M2)	0.641691	2500	29.6975	0	1.3432

Shear Design for V  $_{\rm u2}$  , V  $_{\rm u3}$ 

	Shear V u kN	Shear V c kN	Shear V <sub>s</sub> kN	Shear V <sub>p</sub> kN	ebar A <sub>sv</sub> /s mm²/m
Major, V <sub>u2</sub>	14.4302	60.0621	27.1402	0	254.94
Minor, V <sub>u3</sub>	47.4888	55.5742	24.5003	46.6095	387.95

Joint Shear Check/Design

	Joint Shear Force kN	Shear V Top	<b>Shear</b> V u,Tot	Shear V c	Joint Area cm²	Shear Ratio
		kN	kN	kN		Unitless
Major Shear, V <sub>u2</sub>	N/A	N/A	N/A	N/A	N/A	N/A
Minor Shear, V <sub>u3</sub>	N/A	N/A	N/A	N/A	N/A	N/A

(1.4) Beam/Column Capacity Ratio

Major	Minor
Ratio	Ratio
N/A	N/A

Additional Moment Reduction Factor k (IS 39.7.1.1)

Ag cm <sup>2</sup>	A sc cm <sup>2</sup>	P uz kN	P b kN	P u kN	K Unitless
805	34.8	2210.9778	301.1202	-67.1618	1

Additional Moment (IS 39.7.1)

	Consider M a	Length Factor	Section Depth (mm)	KL/Depth Ratio	KL/Depth Limit	KL/Depth Exceeded	M a Moment (kN-m)
Major Bending (M 3)	Yes	0.833	350	7.143	12	No	0
Minor Bending (M $_2$ )	Yes	0.833	230	6.975	12	No	0

COLUMN-2

ETABS Concrete Frame Design

IS 456:2000 + IS 13920:2016 Column Section Design





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Column Eleme	ent Details Type	Ductile Frame	Summary)
	in Details Type.	Ductile I fullie	Summary

Level	Element	Unique Name	Section ID	Combo ID	Station Loc	Length (mm)	LLRF
Story6	C1	205	COL 2	DCon21	0	3000	0.888

b (mm)	h (mm)	dc (mm)	Cover (Torsion) (mm)
450	450	60	30

E <sub>c</sub> (MPa)	f <sub>ck</sub> (MPa)	Lt.Wt Factor (Unitless)	f <sub>y</sub> (MPa)	f ys (MPa)
27386.13	30	1	500	500

**Design Code Parameters** 

Axial Force and Biaxial Moment Design For P  $_{u}\,$  , M  $_{u2}\,$  , M u3

Design P <sub>u</sub> kN	esign M <sub>u2</sub> kN-m	esign M <sub>u3</sub> kN-m	Minimum M <sub>2</sub> kN-m	linimum M 3 kN- m	Rebar Area mm²	Rebar % %
151.2296	152.4941	-37.4958	3.0246	3.0246	1806	0.89

Axial Force and Biaxial Moment Factors

	K Factor Unitless	Length mm	Initial Momen kN-m	t Additional Mo kN-m	ment	Minimum Moment kN-m
Major Bend(M3)	0.936723	2500	15.3278	0		3.0246
Minor Bend(M	<i>I</i> (2) 0.915942	2500	60.9976	0		3.0246

Shear Design for V  $_{u2}$  , V  $_{u3}$ 

	Shear V <sub>u</sub> kN	Shear V c kN	Shear V <sub>s</sub> kN	Shear V <sub>p</sub> kN	Rebar A <sub>sv</sub> /s mm²/m
Major, V <sub>u2</sub>	30.3261	95.2886	70.2003	0	498.8
Minor, V <sub>u3</sub>	106.9116	95.2886	70.2003	0	498.8

Joint Shear Check/Design

	Joint Shear Force kN	Shear V Top	Shear V u,Tot	Shear V c kN	Joint Area cm²	Shear Ratio
		KIN	KIN			Unitiess
Major Shear, V <sub>u2</sub>	N/A	N/A	N/A	N/A	N/A	N/A
Minor Shear, V <sub>u3</sub>	N/A	N/A	N/A	N/A	N/A	N/A

(1.4) Beam/Column Capacity Ratio

Major	Minor
Ratio	Ratio
N/A	N/A

Additional Moment Reduction Factor k (IS 39.7.1.1)

Ag cm <sup>2</sup>	A sc cm <sup>2</sup>	P uz kN	P b kN	P u kN	k Unitless
2025	18.1	3410.9703	1161.6527	151.2296	1



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Additional Moment (IS 39.7.1)

	Consider M a	Length Factor	Section Depth (mm)	KL/Depth Ratio	KL/Depth Limit	KL/Depth Exceeded	M a Moment (kN-m)
Major Bending (M <sub>3</sub> )	Yes	0.833	450	5.204	12	No	0
Minor Bending (M $_2$ )	Yes	0.833	450	5.089	12	No	0

COLUMN-3

ETABS Concrete Frame Design

#### IS 456:2000 + IS 13920:2016 Column Section Design



#### Column Element Details Type: Ductile Frame (Summary)

Level	Element	Unique Name	Section ID	Combo ID	Station Loc	Length (mm)	LLRF
Story2	С9	535	COL 3	DCon19	0	3000	1

#### Section Properties

b (mm)	h (mm)	dc (mm)	Cover (Torsion) (mm)
550	550	60	30

#### Material Properties

E c (MPa)	f <sub>ck</sub> (MPa)	Lt.Wt Factor (Unitless)	f <sub>y</sub> (MPa)	f ys (MPa)
27386.13	30	1	500	500

#### Design Code Parameters



#### Axial Force and Biaxial Moment Design For P $_{u}$ , M $_{u2}$ , M $_{u3}$

6	esign P <sub>u</sub> kN	Design M <sub>u2</sub> kN-m	Design M <sub>u3</sub> kN-m	Minimum M 2 kN-m	Minimum M 3 kN-m	Rebar Area mm²	Rebar % %
	86.8263	26.562	237.5714	2.0259	2.0259	2632	0.87

#### Axial Force and Biaxial Moment Factors

	K Factor Unitless	Length mm	nitial Moment kN-m	ional Moment kN-m	mum Moment kN-m
Major Bend(M3)	0.794273	2500	104.6556	0	2.0259
Minor Bend(M2)	0.772917	2500	19.9968	0	2.0259

Shear Design for V  $_{u2}$ , V  $_{u3}$ 

	Shear V u kN	Shear V c kN	Shear V <sub>s</sub> kN	Shear V <sub>p</sub> kN	Rebar A <sub>sv</sub> /s mm²/m
Major, V <sub>u2</sub>	132.9159	136.9317	107.8004	0	609.64
Minor, V <sub>u3</sub>	6.5652	136.9317	107.8004	0	609.64



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Joint Shear Check/Design

	Joint Shear Force kN	Shear V Top kN	Shear V u,Tot kN	Shear V c kN	Joint Area cm²	Shear Ratio Unitless
Major Shear, V <sub>u2</sub>	N/A	N/A	N/A	N/A	N/A	N/A
Minor Shear, V <sub>u3</sub>	N/A	N/A	N/A	N/A	N/A	N/A

(1.4) Beam/Column Capacity Ratio

Major	Minor
Ratio	Ratio
N/A	N/A

Additional Moment Reduction Factor k (IS 39.7.1.1)

A g cm²	A sc cm <sup>2</sup>	P uz kN	P b kN	P u kN	k Unitless
3025	26.3	5070.7326	1833.9532	86.8263	1

Additional Moment (IS 39.7.1)

	Consider M a	Length Factor	Section Depth (mm)	KL/Depth Ratio	KL/Depth Limit	KL/Depth Exceeded	M a Moment (kN-m)
Major Bending (M <sub>3</sub> )	Yes	0.833	550	3.61	12	No	0
Minor Bending (M $_2$ )	Yes	0.833	550	3.513	12	No	0

#### 5. CONCLUSION

G+3-storey apartment building analysis and design. ETABS V13.2, a premium section analysis and design programme, is used. RCC frame, shear wall, and retaining walls. The soil investigation report gives an isolated footing. ETABS developed RCC beams and columns. Analysis and design were as conventional as possible. Recognized were the structural engineer's design problems.

Manual design and software analysis data for structures Following conclusions are drawn:

1.ETABS was used to analyse and manually verify IS456.

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